

## Engineering properties of lightweight aggregate concrete made from dredged silt

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### Abstract

Silt dredged from reservoirs can be hydrated and sintered into lightweight aggregate for producing lightweight aggregate concrete (LWAC). The densified mixture design algorithm (DMDA) was employed to manufacture LWAC using 150 kg/m<sup>3</sup> of water at different water-to-binder ratios ( $w/b = 0.28, 0.32$  and  $0.4$ ) using lightweight aggregates of different particle densities (800, 1100 and 1500 kg/m<sup>3</sup>). The engineering properties of the LWAC thus obtained were examined. Results show that the fresh concrete meets the design requirement of having slump of  $250 \pm 20$  mm and slump flow of  $600 \pm 100$  mm. With respect to hardened properties, the compressive strength, ultrasonic pulse velocity and thermal conductivity were found to decrease with increasing  $w/b$  ratio but increase with increasing aggregate density. Moreover, higher aggregate density also resulted in less shrinkage. The surface resistivity exceeding 20 k $\Omega$ -cm also matched the design objective. The experimental results prove that LWAC made from dredged silt can help enhance durability of concrete.

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### 1. Introduction

Accumulation of silt is a common problem of reservoirs in Taiwan resulting in annual decrease in water storage capacity. While it is unlikely that new reservoirs will be constructed in the near future, the government has stepped up its effort in dredging the silt with the intention to maintain a stable supply of water and extending the service life of the reservoirs [1]. Nevertheless, improper disposal or treatment of the silt dredged may have detrimental effects on the environment and arouse public discontent. Taiwan, being densely populated and having limited natural resources, has developed technologies for the reuse of dredged silt from the reservoirs [2]. Since sands and stones needed for cement mixing are in short supply, reuse of

dredged silt offers an economic and ecological alternative. Through hydration and high-temperature sintering, dredged silt is made into lightweight aggregate, which is then used to produce high-performance lightweight aggregate concrete (LWAC). Although there has been abundant and fruitful research carried out on the development and manufacturing techniques for LWAC [3], its application to engineering or public construction is less common than high-performance or traditional concrete [4]. In fact, LWAC is a very versatile material for construction. It offers a range of technical, economic and environment-enhancing and preserving advantages. Its properties have been widely studied [5] and its strength and durability have been proved to be good [6]. Compared with its normal-weight counterpart, lightweight aggregate has higher water absorption rate and lower relative density. In addition to being light, it has good strength, fire resistance and heat insulation. LWAC has been used in buildings and bridges to reduce the dead weight and the dimensions of structural

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members. This research aims to study the properties of LWAC mixed using lightweight aggregates made from sintered silt dredged from reservoirs. Moreover, lightweight aggregates of different densities and various water-to-binder (w/b) ratios were used to examine their influence on engineering properties of LWAC.

## 2. Experimental study

### 2.1. Materials

The lightweight aggregate manufactured from dredged silt from reservoirs is a synthetic aggregate. Materials used in this study for making lightweight aggregate concrete include the ASTM C 150 compliant Type I ordinary Portland cement (OPC), fly ash from a local factory, blast-furnace slag from Chung Lien Factory, aggregates and natural sands according to ASTM C33, and Type 1000 superplasticizer which complies with the ASTM C 494 type G admixture. The chemical composition and physical properties of these materials are shown in Tables 1–3. The maximum size of LWA (1) used in the mixes is 20 mm and LWA (2) and LWA (3) are 13 mm.

### 2.2. Test variables

The densified mixture design algorithm (DMDA) was employed to mix the LWAC. In this study, the concrete was filled using 60% of lightweight aggregate and fly ash to replace 15% of sand required, slag to replace 5% of cement. With the basic materials and their chemical compositions known, the density of the lightweight aggregate

Table 2

Physical properties of aggregates

| Physical properties                   | Coarse aggregate (>4 mm) |         |         | Fine aggregate (<4 mm) |
|---------------------------------------|--------------------------|---------|---------|------------------------|
|                                       | LWA (1)                  | LWA (2) | LWA (3) | Natural sand           |
| Particle density (kg/m <sup>3</sup> ) | 800                      | 1100    | 1500    | 2630                   |
| Absorption capacity (24 h) (%)        | 9.6                      | 8.0     | 4.2     | 0.9                    |
| Max. size $D_{\max}$ (mm)             | 20                       | 13      | 13      | 2.4                    |
| Fineness modulus (FM)                 | 6.95                     | 6.48    | 6.40    | 2.88                   |
| Unit weight (kg/m <sup>3</sup> )      | 498                      | 658     | 836     | 1650                   |
| Compressive strength (MPa)            | 2.66                     | 3.33    | 6.74    | –                      |

Table 3

Basic properties of superplasticizer

| Properties                            | Type 1000 superplasticizer |
|---------------------------------------|----------------------------|
| Solid ingredient (%)                  | 43.0                       |
| Chloride ion content (ppm)            | 50.1                       |
| Insoluble residue (%)                 | 0.15                       |
| Specific gravity (g/cm <sup>3</sup> ) | 1.18                       |
| pH value                              | 6.93                       |

and w/b ratio were varied to examine their effect on the properties and compressive strength of the fresh concrete. Lightweight aggregate of three different particle densities, namely 800, 1100 and 1500 kg/m<sup>3</sup>, and three w/b ratios, namely 0.28, 0.32 and 0.40 were used. Table 4 shows the nine mix proportions for preparing LWAC.

### 2.3. Test measurements

The concrete was mixed according to the specifications of ACI 211.4R. The DMDA was employed to achieve the maximum unit weight while minimizing internal porosity [7]. The slump and slump flow of the fresh concrete were determined in accordance with ASTM C143. The test specimens for compressive strength were 100 × 200 mm cylinders as per ASTM C31 and C39, while the pulse velocity was measured according to ASTM C597. Electrical resistance coefficient of LWAC was measured by the concrete resistivity meter (CNS Electronics Ltd.). Specimens of 40 × 200 × 200 mm were made according to DIN 51046 for measuring the coefficient of thermal conductivity on the surface at 10 °C and 40 °C using a quick thermal conductivity meter (QTM-D2). The drying shrinkage specimens of 101.6 × 101.6 × 279.4 mm were demolded 24 h after casting, and cured under water at a temperature of 23 ± 2 °C for 28 days. The above measurements were taken to examine the effect of lightweight aggregate density and w/b ratio on the fresh and hardened properties of the LWAC, which would also shed light on the workability, homogeneity and durability of LWAC.

Table 1

Chemical composition and physical properties of cement, fly ash and slag

| Items                                 | OPC   | Fly ash (Type F) | Slag  |
|---------------------------------------|-------|------------------|-------|
| <i>Chemical composition (wt.%)</i>    |       |                  |       |
| SiO <sub>2</sub> (S)                  | 22.01 | 51.23            | 34.86 |
| Al <sub>2</sub> O <sub>3</sub> (A)    | 5.57  | 24.31            | 13.52 |
| Fe <sub>2</sub> O <sub>3</sub> (F)    | 3.44  | 6.14             | 0.25  |
| S+A+F                                 | 31.02 | –                | 48.63 |
| CaO (C)                               | 62.80 | 6.28             | 41.77 |
| MgO (M)                               | 2.59  | 1.61             | 7.18  |
| SO <sub>3</sub> (S)                   | 2.08  | 0.61             | 1.74  |
| f-CaO                                 | 1.05  | –                | –     |
| TiO <sub>2</sub> (T)                  | 0.52  | 1.42             | –     |
| Na <sub>2</sub> O (Na)                | 0.40  | 0.21             | –     |
| K <sub>2</sub> O (K)                  | 0.78  | 1.16             | –     |
| V <sub>2</sub> O <sub>5</sub> (V)     | 0.05  | –                | –     |
| Loss on ignition (LOI)                | 0.51  | 4.85             | 0.31  |
| <i>Physical properties</i>            |       |                  |       |
| Fineness (m <sup>2</sup> /kg)         | 297   | 311              | 435   |
| Specific gravity (g/cm <sup>3</sup> ) | 3.15  | 2.19             | 2.87  |
| Initial setting time (h:min)          | 1:25  | –                | –     |
| Final setting time (h:min)            | 2:31  | –                | –     |
| 28-day mortar cube strength (MPa)     | 56    | –                | –     |
| Retention on 325 sieve (%)            | –     | –                | 8.0   |

Table 4  
Mix proportions of lightweight aggregate concrete

| Particle density ( $\rho$ ) (kg/m <sup>3</sup> ) | Batch    | w/b  | w/c  | Materials (kg/m <sup>3</sup> ) |        |        |         |      |       |    |                    |             |
|--|----------|------|------|--------------------------------|--------|--------|---------|------|-------|----|--------------------|-------------|
|  |          |      |      | Sand                           | L. agg | Cement | Fly ash | Slag | Water | SP | $W_{\text{total}}$ | Unit weight |
| 800  | U0828150 | 0.28 | 0.39 | 751                            | 283    | 383    | 133     | 20   | 123   | 27 | 150                | 1720        |
|  | U0832150 | 0.32 | 0.48 | 774                            | 292    | 315    | 137     | 17   | 134   | 16 | 150                | 1685        |
|  | U0840150 | 0.4  | 0.68 | 807                            | 304    | 221    | 143     | 12   | 144   | 6  | 150                | 1637        |
| 1100   | U1128150 | 0.28 | 0.39 | 751                            | 426    | 383    | 133     | 20   | 125   | 25 | 150                | 1863        |
|  | U1132150 | 0.32 | 0.48 | 774                            | 440    | 315    | 137     | 17   | 138   | 12 | 150                | 1833        |
|  | U1140150 | 0.4  | 0.68 | 807                            | 459    | 221    | 143     | 12   | 144   | 6  | 150                | 1792        |
| 1500   | U1528150 | 0.28 | 0.39 | 751                            | 570    | 383    | 133     | 20   | 126   | 24 | 150                | 2007        |
|  | U1532150 | 0.32 | 0.48 | 774                            | 588    | 315    | 137     | 17   | 138   | 12 | 150                | 1981        |
|  | U1540150 | 0.4  | 0.68 | 807                            | 613    | 221    | 143     | 12   | 144   | 6  | 150                | 1946        |
| 1100 (ACI)                                       | U1128223 | 0.28 | 0.28 | 417                            | 409    | 796    | 0       | 0    | 200   | 23 | 223                | 1845        |
|  | U1132223 | 0.32 | 0.32 | 504                            | 409    | 697    | 0       | 0    | 223   | 0  | 223                | 1833        |
|  | U1140223 | 0.4  | 0.4  | 625                            | 409    | 558    | 0       | 0    | 223   | 0  | 223                | 1815        |

b: Binder including cement, fly ash and slag; SP: Type 1000 superplasticizer ASTM C494 HRWRA, naphthalene-based; SP (%): weight of superplasticizer to binder content ratio; W: clean water, including water in the superplasticizer.

### 3. Test results and discussion

#### 3.1. Properties of fresh concrete

Table 5 shows the properties of fresh concrete mixed with 150 kg/m<sup>3</sup> of total water using LWAC of various densities at different w/b ratios. The initial slump of the fresh concrete was found to be  $250 \pm 20$  mm and the slump flow was  $600 \pm 100$  mm. Even after 60 min, the workability of the fresh concrete was maintained, showing neither water bleeding nor segregation of aggregate.

#### 3.2. Compressive strength

Fig. 1 shows the relationship between compressive strength and aggregate density at different w/b ratios. As expected, there is enhancement in compressive strength with higher aggregate density and lower w/b ratio. However, the differences in compressive strength become less

significant with longer curing as evidenced by the smaller increase after 56 days of curing. This reduction in strength development is constrained by the strength of the lightweight aggregate.

#### 3.3. Ultrasonic pulse velocity

Fig. 2 shows the variation of ultrasonic pulse velocity and aggregate density and therefore compressive strength at different w/b ratios. As seen in the figure, the pulse velocity of concrete of different mix proportions all reached 3900 m/s after 90 days of curing.

#### 3.4. Electrical resistivity

Fig. 3 shows the changes in electrical resistivity of LWAC with different w/b ratios. As expected, the lower the w/b ratio, the higher the electrical resistance. Low w/b ratio resulting in greater durability of concrete is also

Table 5  
Properties of fresh LWAC

| Particle density ( $\rho$ ) (kg/m <sup>3</sup> ) | Batch    | Initial    |                 |               | At 60 mins |                 |               |
|--|----------|------------|-----------------|---------------|------------|-----------------|---------------|
|  |          | Slump (mm) | Slump flow (mm) | Flow time (s) | Slump (mm) | Slump flow (mm) | Flow time (s) |
| 800  | U0828150 | 270        | 615             | 190           | 265        | 610             | 195           |
|  | U0832150 | 260        | 610             | 150           | 255        | 595             | 180           |
|  | U0840150 | 260        | 560             | 115           | 250        | 540             | 145           |
| 1100   | U1128150 | 275        | 670             | 115           | 270        | 660             | 120           |
|  | U1132150 | 265        | 615             | 160           | 260        | 600             | 180           |
|  | U1140150 | 260        | 650             | 40            | 250        | 630             | 60            |
| 1500   | U1528150 | 270        | 685             | 150           | 260        | 630             | 170           |
|  | U1532150 | 255        | 560             | 135           | 240        | 520             | 180           |
|  | U1540150 | 245        | 650             | 70            | 235        | 550             | 100           |
| 1100 (ACI)                                       | U1128223 | 130        | —               | —             | 50         | —               | —             |
|  | U1132223 | 105        | —               | —             | 55         | —               | —             |
|  | U1140223 | 150        | —               | —             | 110        | —               | —             |

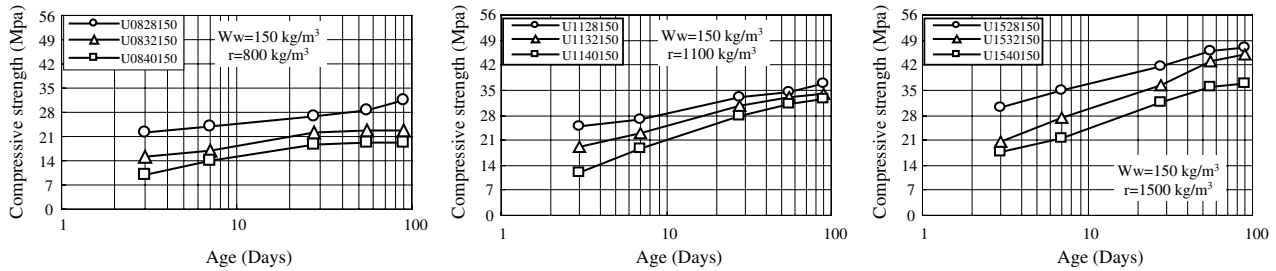


Fig. 1. Relationship between compressive strength and aggregate density at different w/b ratios.

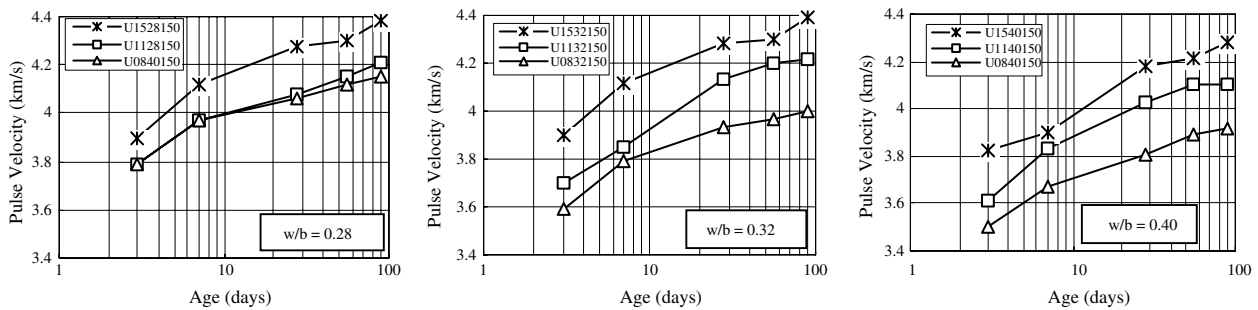


Fig. 2. Relationship between ultrasonic pulse velocity and aggregate density at different w/b ratios.

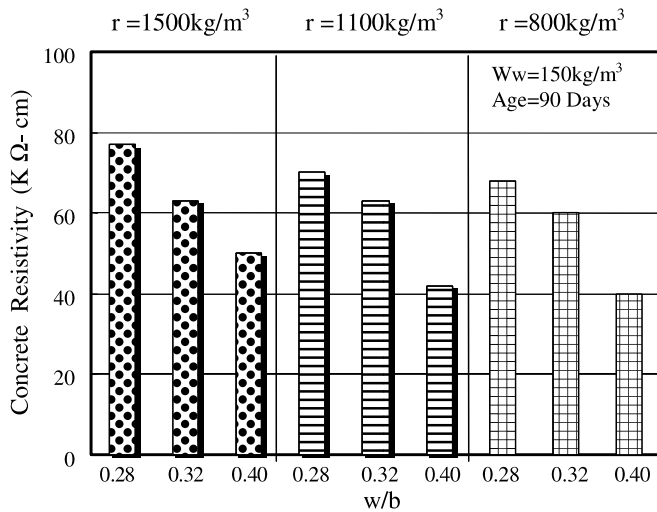


Fig. 3. Relationship between electrical resistivity with different aggregate densities and w/b ratios after 90 days of curing.

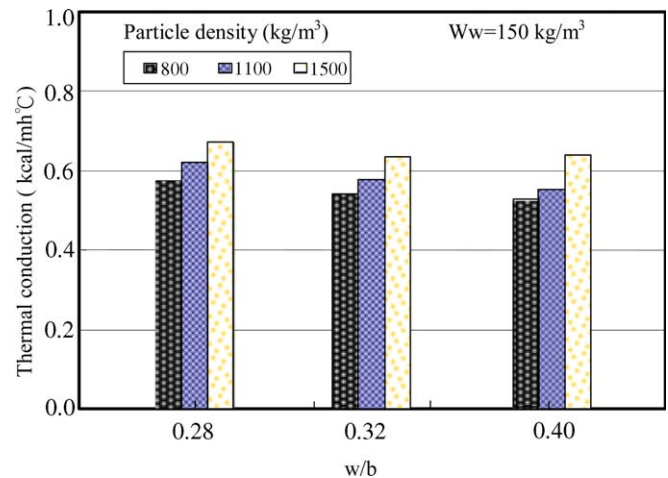


Fig. 4. Relationship between thermal conductivity coefficient with different aggregate densities and w/b ratios.

evidenced by the better compressive strength and higher ultrasonic pulse velocity. For the same w/b ratio, aggregate of greater density will increase electrical resistivity. The electrical resistivity of the LWAC reached above 40 kΩ-cm after 90-day curing.

### 3.5. Thermal conductivity coefficient

Fig. 4 shows the thermal conductivity coefficient under different aggregate densities and w/b ratios. As expected, thermal conductivity of the concrete is related to the appar-

ent density: the lower the aggregate density, the smaller the thermal conductivity coefficient will be. In this study, the effect of different w/b ratios on thermal conductivity was found to be not significant because the amounts of aggregate used are almost the same. The coefficient of LWAC, being 0.5–0.7 kcal/m h °C, is much smaller than that of normal-weight concrete (1.0–1.5 kcal/m h °C), implying better heat insulation of LWAC.

### 3.6. Shrinkage

Table 6 shows the expansion at 28 days and the shrinkage at 90 days. The results show on expansion varying from

Table 6  
Expansion and shrinkage of LWAC (w/b = 0.32)

| Particle density ( $\rho$ ) | Batch    | Expansion at 28 days | Shrinkage at 90 days |
|-----------------------------|----------|----------------------|----------------------|
| 800                         | U0832140 | 110                  | 450                  |
|                             | U0832150 | 130                  | 462                  |
|                             | U0832160 | 150                  | 510                  |
| 1100                        | U1132140 | 100                  | 390                  |
|                             | U1132150 | 120                  | 500                  |
|                             | U1132160 | 144                  | 510                  |
| 1500                        | U1532140 | 110                  | 410                  |
|                             | U1532150 | 110                  | 418                  |
|                             | U1532160 | 110                  | 430                  |

100 to  $150 \times 10^{-6}$  whereas at 90 days the shrinkage varied from 390 to  $510 \times 10^{-6}$ .

#### 4. Conclusions

From the study reported here, the following conclusions can be drawn.

1. The DMDA has proved to be capable of producing LWAC mixes with a slump of 230 mm and slump flow above 500 mm while avoiding bleeding and segregation.
2. With a particle density of  $800 \text{ kg/m}^3$ , the 28 day compressive strength reached 20–30 MPa; while it reached 30–40 MPa at particle density of 1100 and  $1500 \text{ kg/m}^3$ .
3. The LWAC reported here developed a pulse velocity above 3900 m/s after curing for 90 days.

4. LWAC mixes using the DMDA has better electrical resistivity, above  $40 \text{ k}\Omega\text{-cm}$  after 90-day curing.
5. The thermal conductivity coefficient of LWAC ranged between 0.5–0.7 kcal/m h °C, which is far smaller than that of normal-weight concrete.
6. Expansion after 28 day water curing reached a maximum of around  $150 \times 10^{-6}$ , while the maximum shrinkage at 90 days was  $510 \times 10^{-6}$ .

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